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Article

Quantification of the Post-Fire Strength Retention Factors for Selected Standard Duplex and Lean Duplex Stainless Steel Grades

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Abstract: Experimental quantification of retention factors related to post-fire strength as well as post-fire ductility of intentionally selected stainless steel grades applied in construction is the objective of research presented here. These steel grades are characterized by a two-phase austenitic-ferritic microstructure of the duplex type. In this context two mutually corresponding chromium-nickel-molybdenum steel grades have been subjected to analysis, namely X2CrNiMoN22-5-3 steel belonging to the standard duplex group (DSS 22% Cr) and X2CrMnNiN21-5-1 steel belonging to the lean duplex group (LDSS). The similarities and differences in the mechanical properties exhibited by these steel grades after effective cooling, following more or less prolonged simulated fire action conforming to several development scenarios, have been identified and indicated. The resistance of given steel grade to permanent structural changes induced by the heating program proved to be the critical factor determining these properties and resulting in many cases in increased susceptibility to brittle fracture.

Keywords: stainless steel; standard duplex steel; lean duplex steel; post-fire strength; post-fire ductility; retention factors; recovery factors

1. Introduction—Standard duplex versus lean duplex stainless steels used in construction

Duplex steels have a special place in modern construction industry, where stainless steels are used in wide range of applications. Their intentionally designed and properly balanced two-phase microstructure, comprising of face-centered austenite and body-centered ferrite in approximately equal ratio and crystallizing in a cubic system, determines relatively high strength accompanied by manufacturer warranted ductility, as expected by the user [1]. The basic difference in the chemical composition of these steels, when compared against the chemical composition typical for conventional stainless steels exhibiting austenitic structure, is substantially higher chromium content by weight (usually between 20-28%) and increased molybdenum content (up to 5%) [2]. At the same time nickel content is by a rule decreased to 9% while nitrogen content remains at 0.05-0.50% [3]. Due to the very high corrosion resistance, in particular to the pitting corrosion initiated in the chlorine rich environments, these steels find wide application among others in petrochemical industry, for instance in pipeline construction or construction of pressurized storage tanks [4].

The chemical composition of duplex steels is usually driven by the requirement to obtain thermodynamic stability of their most substantial components, i.e. austenite and ferrite, and, independently, nitrogen limit solubility. On the one hand, when the combined chromium and molybdenum content by weight in the chemical composition does not exceed 20% one has to take into account the risk of martensitic transformation of austenite. This, by a rule, results in hardening

of the material and as a consequence significant deterioration in ductility. Transition of this type is usually related to high susceptibility to brittle failure, occurring in an abrupt fashion, without any forewarning of gradually increasing failure risk. On the other hand, when the chromium and molybdenum content by weight in the designed steel exceeds 35%, the δ -ferrite instability may occur, accompanied at sufficiently high temperature by secondary precipitates, deleterious to its mechanical properties. High pressure of nitrogen dissolved under these conditions in the structure of such steel may constitute an additional source of instabilities generated between individual phases [5].

As is widely known, the stainless steels of duplex type, depending on the intentionally selected chemical composition may be divided into five basic qualitative groups. These are as follows:

1. Lean Duplex Stainless Steels - LDSS,
2. Standard Duplex Stainless Steels (with 22% Cr content) - DSS 22% Cr,
3. High Alloyed Standard Duplex Stainless Steels (with 25% Cr content) - DSS 25% Cr,
4. Super Duplex Stainless Steels – SDSS [6,7],
5. Hyper Duplex Stainless Steels - HDSS.

The stainless steels belonging to the LDSS group, when compared against stainless steels belonging to the DSS 22%Cr group, are characterized by the significantly lowered nickel and molybdenum content in their chemical composition. Reduction in the content of these elements, applied to stabilize austenite, requires an addition of manganese. This addition increases solubility of nitrogen in the solution, and as a consequence increases the resistance of these steel grades to pitting corrosion [8]. However, in the case of LDSS steels this resistance is significantly lower than resistance exhibited by the steels belonging to the DSS 22% Cr group. The LDSS steels have been designed out of necessity as a cheaper substitute of the DSS 22% Cr steels, and have been introduced on the market driven by wide fluctuations in the price of hardly available nickel.

2. Steel grades selected for detailed analysis

The research presented in this paper deals with two, subjectively selected and juxtaposed for comparative purposes, stainless steel grades exhibiting two-phase austenitic-ferritic structure of duplex type. The first of these grades is X2CrNiMoN22-5-3 stainless steel belonging to the standard duplex group (DSS 22% Cr). Following its classification, this is a high-alloy, chromium-nickel-molybdenum steel. In the commercial nomenclature this steel is denoted as 1.4462 (Werkstoffnummer). The second grade is X2CrMnNiN21-5-1 stainless steel, also a chromium-nickel-molybdenum one, but counted among the lean duplex steels (LDSS). In the commercial nomenclature this steel is denoted as 1.4162 (Werkstoffnummer). The properties of both these steels are listed in detail in the code EN 10088-1 [9]. Their chemical composition is listed in the Table 1.

One may easily note, that the LDSS steel selected for analysis, when compared against the DSS 22% Cr steel, is characterized by significantly decreased nickel and molybdenum content. At the same time it is characterized by substantially increased manganese content.

Both stainless steel grades considered in this paper exhibit comparable mechanical properties when analyzed at room temperature. This in particular refers to the $R_{0.2}$ yield limit and ultimate strength R_m . These are $R_{0.2} > 500 \text{ MPa}$ and $R_m = 660\text{-}950 \text{ MPa}$ in the case of DSS 22% Cr steel as well as $R_{0.2} > 480 \text{ MPa}$ and $R_m = 650\text{-}850 \text{ MPa}$ in the case of LDSS steel, respectively.

Table 1. Chemical composition of steels selected for detailed analysis [% by weight] (according to [9]).

Element	X2CrNiMoN22-5-3 (DSS 22% Cr)	X2CrMnNiN21-5-1 (LDSS)
C	<0.03	<0.04
Si	<1.0	<1.0
Mn	<2.0	4.0-6.0
P	<0.035	<0.040
S	<0.015	<0.015
N	0.10-0.22	0.20-0.25
Cr	21.0-23.0	21.0-22.0
Mo	2.5-3.5	0.10-0.80
Ni	4.5-6.5	1.35-1.70
Cu	-	0.10-0.80

3. The purpose and scope of conducted research

Our research was oriented on testing the post-fire mechanical properties exhibited by the compared steel grades. It is widely known, that every steel grade subjected to the action of fire, after cooling does not revert to its initial internal structure, and therefore its properties differ substantially from those exhibited prior to the fire incident. Behavior of stainless steels subjected to the simulated fire tests usually differs from the behavior exhibited by common structural steels, and in particular low-carbon steel grades [10,11]. The peculiarity of this behavior is determined by permanent changes occurring in the microstructure of tested material when subjected to the action of high temperature and remaining after cooling [12]. However, the way these changes occur, and in particular their intensity and probability of initiation depend to a high degree on the characteristics of fire affecting considered steel. The key here is not only the temperature to which the steel has been heated or the heating speed [13], but also the time during which the material was subjected to the action of constant high temperature [14]. The situation changes diametrically, if only under those circumstances the steel reached a temperature sufficient for unrestrained initiation of austenitic transformation, accompanied by all more or less important repercussions resulting from the transformation of this type. The tests conducted so far indicated strong dependence of post-fire material properties of considered steels on the cooling mode applied at the end of tests [15]. When rapid cooling was applied, by voluminous application of water spray to simulate a fire extinguishing action, the cooled material, due to local hardening, may prove to be very susceptible to brittle failure.

The first stage of testing conducted was oriented on identification of such fire scenarios, which could, after application to duplex steel grades selected for detailed analysis, result in unacceptable brittleness of the material [16]. To this purpose a wide program of experimental tests has been conducted, based on series of instrumented Charpy impact tests planned and adjusted to post-fire conditions. These tests have been conforming to European [17,18] and to US [19,20] standards. The results of these tests have been in part published and were presented at first in [21,22], relating to the credibility of the testing method itself and the conclusions drawn after application of the selected testing methodology, and later on in [23–27], taking into account various aspects of final rating.

During research conducted so far the heating level of tested samples has been selected intentionally so as to enable the initiation of deleterious phase changes in the structure of considered material, resulting in decreased ductility. The research was oriented on discerning the sensitivity of tested duplex steels to various types of precipitations related to these changes. The results presented in [28] have been applied for this purpose. In particular, a more or less extended time of passing through two temperature ranges was of particular interest here:

- a 475°C brittleness zone – related to partial change of δ -ferrite into spinoidal secondary α' -ferrite, and precipitation of π , ε and G brittle phases (in the steels tested here this phenomenon occurs in the temperature range of 300-550°C),
- a 800°C brittleness zone – induced by precipitation from the solid solution (mostly δ -ferrite) of carbides M_7C_3 and $M_{23}C_6$, nitrides Cr_2N as well as secondary phases σ , χ , R and γ_2 (in the steels tested here this phenomenon occurs in the temperature range of 600-1050°C).

The decreased molybdenum content by weight, characteristic for LDSS steels (with respect to analogous content typical for steels belonging to the DSS 22% Cr group), should result in this context in [29]:

- increased upper threshold initiation temperature limit for 475°C brittleness phenomenon,
- decreased lower threshold initiation temperature limit for 800°C brittleness phenomenon.

The second part of these tests completing the abovementioned tests dealing with verification of post-fire brittleness, dealt with determination of post-fire mechanical properties of analyzed steels, in particular the yield limit and the tensile strength. The detailed results obtained are presented in the following part of this paper. These results seem to confirm the peculiar behavior of these steels, exhibiting two-phase austenitic-ferritic structure of duplex type, determined in fire conditions. In this context it qualitatively differs from the one exhibited by stainless steels of purely austenitic structure

[11] as well as from the one which seems to be typical of analogous steels exhibiting purely ferritic structure [30].

Tested samples in the case of both tested steel grades have been taken from hot rolled steel plates. Thus qualitative interpretation of numerical results obtained here should be restricted to the material subjected to this type of plastic processing. Therefore the alternative scenario of testing post-fire mechanical properties of steels subjected to cold forming is not being dealt with here. It is well known, that the plastic processing method, determining the mechanical properties of steel prior to a fire episode affects its properties determined post-fire and after cooling as well. This is discussed, among others, in papers [31,32].

4. Sample preparation method and conduct of tests

Experimental verification of the yield limit $f_{y,\Theta}^{post}$ and ultimate strength limit $f_{u,\Theta}^{post}$ exhibited by duplex steels after simulated fire incident and selected for detailed analysis has been conducted on the strength testing machine WDW-300E, capable of generating maximum tensile force of 300 kN (Figure 1). The lower index Θ denotes here an earlier action of fire temperature on the tested specimen, while the upper index denotes that indicated quantity has been determined on the sample effectively cooled after surviving a simulated fire incident. The tested "fivefold" samples of normalized shape and dimensions [33] (Figure 2), have been cut from hot rolled steel plates.



Figure 1. Testing setup for static tension testing of samples cooled after surviving a simulated fire incident.

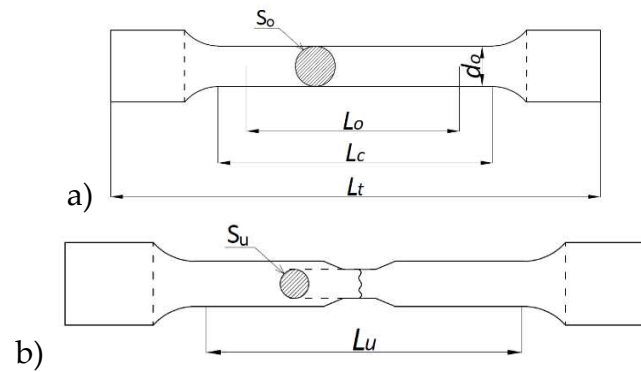


Figure 2. Normalized sample used for testing: a) sample shape prior to rupture, b) sample shape after rupture.

Software included with the strength testing machine is capable of registering relations of various types, and in particular those displaying the load – time, extension - time, load - extension, load – displacement or stress – relative displacement relations. Prior to each test an initial base length L_0 equal to 40 mm has been marked on each sample with a scribing device with intermediate points placed every 5 mm (Figure 3). An extensometer with base length of 25 mm, mounted on the sample as shown in Figure 5, had been used to measure the elongation. This extensometer has been removed from the sample at the initiation of plastic deformation. After conclusion of test (i.e. after breaking the sample) the final length L_u (Figure 2) has been measured with an electronic caliper (Figure 5).



Figure 3. Marking of the initial (base) measurement length on a sample.

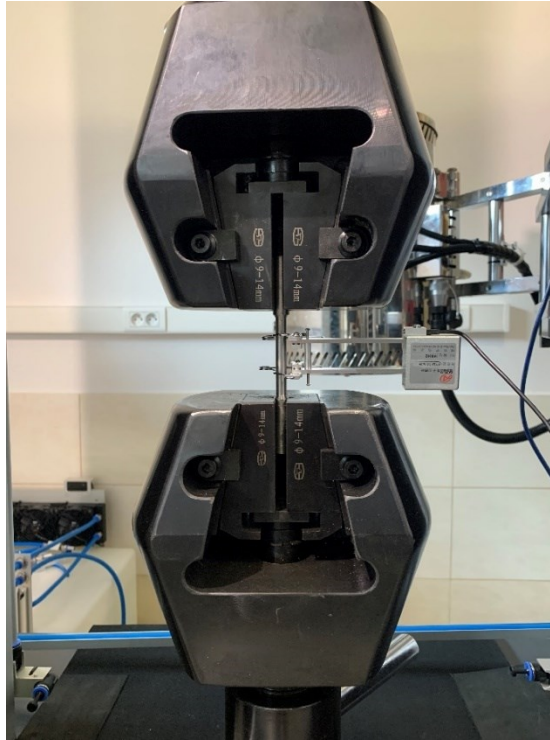


Figure 4. A sample in the grips of the strength testing machine. Extensometer mounting is visible.

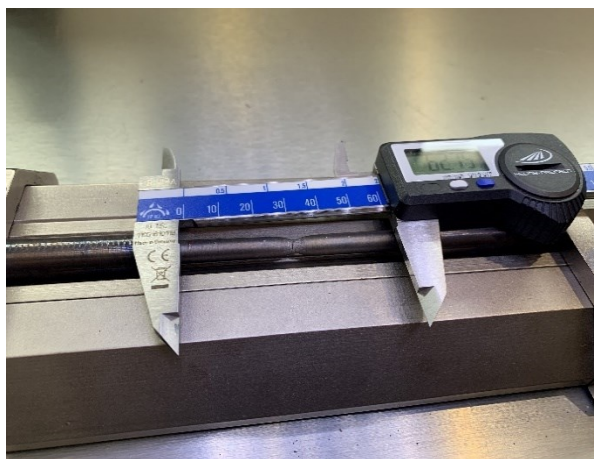
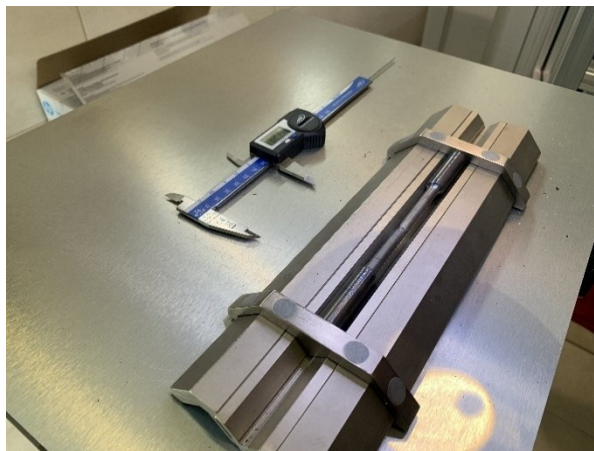


Figure 5. Equipment used to determine relative elongation on a broken sample.

As indicated above, the tested samples have been cooled after prior action of a simulated fire incident (Figure 6). Within the scope of this simulation several fire scenarios have been considered, each time modeled following the isothermal testing mode (the so called *steady-state heating regime*). In particular, the following scenarios have been used: a “short” fire with the sample kept for one hour at the fire temperature, and a “long” fire, where the time spent by the sample at the fire temperature has been extended to ten hours. The tested samples have been heated up to the predetermined testing temperature, equal to 600°C in the first series of tests and 800°C in the second series with constant speed of 100°C/min (Figure 7). These heating levels have been selected intentionally, as the first of these levels is believed to be too low while the second is considered to be sufficiently high to induce during the tests conducted on samples made of mild carbon steel thermally generated permanent structural changes, determining their post-fire strength [12]. It has been decided to preserve those temperature levels during the tests conducted on samples made of stainless steels of the duplex type for the comparability reasons, to preserve the capability to compare the results obtained during research reported here with the results of other tests.



Figure 6. KJ-M1200-64L-IC muffle furnace used in the experiment to heat the tested samples in simulation of a fire incident.

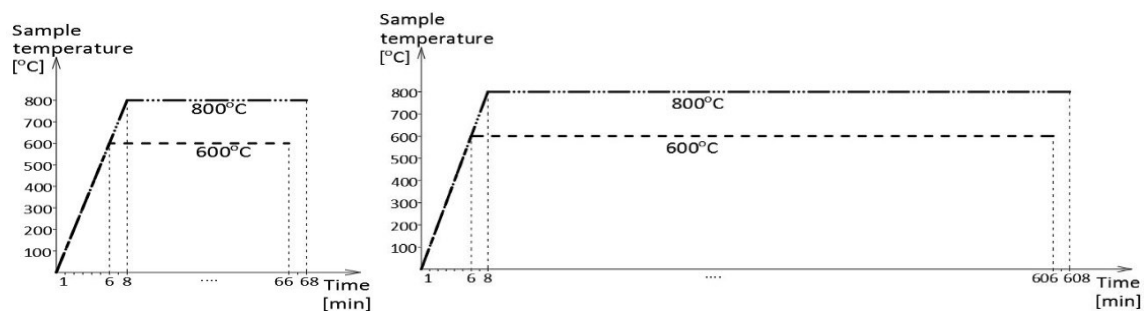


Figure 7. Fire simulation scenarios followed during the experiments: “short” fire at left, “long” fire at right.

Different sample cooling modes have been applied during the tests as well. In one group of tests the samples were cooled slowly inside the laboratory furnace, to simulate the self-extinguishing of fire, while in the other group the samples have been cooled rapidly, in water mist, to simulate the fire extinguishing action conducted by fire fighters.

Therefore 18 samples have been subjected to tensile strength testing (2 steel grades times 2 fire simulation scenarios times 2 cooling modes applied plus for each steel grade one so called reference sample made of the material not subjected to the action of simulated fire temperature) [34].

Chemical composition of samples analyzed here, made of duplex stainless steel as described above, has been positively verified against the composition listed in the code and presented in the

Table 1, using the *Foundry-Master* optical emission spectrometer made by Worldwide Analytical Systems, Uedem, Nordrhein-Westfalen, Germany.

5. The results obtained and their interpretation

5.1. Sample description mode applied

The results obtained during the tests are presented here on the stress – strain graphs prepared separately for each tested steel grade.

The following four character key comprising of digits and letters has been used to describe particular samples tested:

- first character (digit 1 or 2) – denotes steel grade tested (1 – X2CrNiMoN22-5-3 steel, 2 – X2CrMnNiN21-5-1 steel, respectively),
- second character (digit 6 or 8) – denotes sample heating level (6 – 600°C or 8 – 800°C, respectively),
- third character (letter F or W) – denotes sample cooling mode after simulated fire incident (F – slow cooling inside the laboratory furnace or W – rapid cooling in water mist, respectively),
- fourth character (letter X or Y) – denotes fire simulation scenario applied (X – “long” fire duration or Y – “short” fire duration, respectively).

The σ - ε curve denoted with single digit instead of four letter code refers to the sample made of X2CrNiMoN22-5-3 steel when marked with 1, while curve denoted with 2 refers to the sample made of X2CrMnNiN21-5-1 steel and is a typical reference curve, since it has been determined while testing a sample made of an as delivered material, i.e. not subjected to thermal action simulating a fire incident. Since such sample did not undergo heat treatment it was not subjected to cooling.

5.2. Results obtained for the DSS 22% Cr X2CrNiMoN22-5-3 steel

The resultant σ - ε curves obtained on samples made of X2CrNiMoN22-5-3 steel cooled down after surviving a simulated fire incident are depicted in Figure 8 – for samples subjected to the simulated “short” fire and in Figure 9 – for samples subjected to the simulated “long” fire.

One may easily observe on presented graphs, that after surviving a “short” fire the X2CrNiMoN22-5-3 steel cooled slowly in the furnace exhibits significant strengthening. This phenomenon may be particularly visible on the samples subjected to heating at the temperature of 600°C (the sample 16FX). It has to be noted however, that this strengthening of material, related to partial hardening, is accompanied by simultaneous and quite significant reduction in ductility. This ductility proved to be substantially higher when the sample heated following the same heating program has been cooled down rapidly in water mist (the sample testing scenario denoted as 16WX). On one hand, the sample heating was too low to initiate in the material precipitation of the deleterious phases related to the 800°C brittleness zone, while on the other hand, at sufficiently high speed of cooling, the transition time through the 475°C brittleness zone was relatively short. Therefore, under those conditions, high ductility was not accompanied by simultaneous strengthening of the material, as it cooled down too fast to harden. One may observe here even slight post-fire weakening of the material, if this conclusion is related to lower values of tensile stresses and relatively low magnitude of strains (sample 16WX).

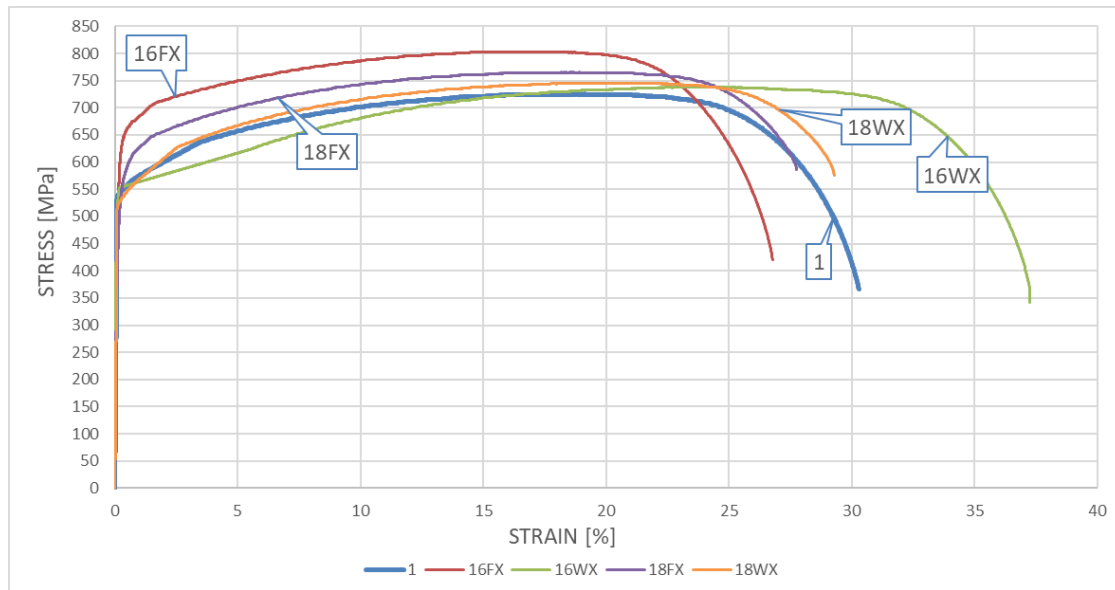


Figure 8. The σ - ε relationship obtained on samples made of $X2CrNiMoN22-5-3$ steel – a “short” fire scenario.

The testing scenario related to heating the steel at 800°C yielded qualitatively different results. Under this scenario the material cooling method selected seemed to have significantly smaller effect. One may compare here the graphs identified as 18FX and 18WX. Ductility of tested steel determined after fire in general remained the same as before. The material strengthening observed post-fire proved to be quantitatively insignificant as well. It seems, that the qualitative differences observed here with respect to the same steel heated for the same “short” time, but at a lower temperature (only 600°C), may be attributed to the hot material entering in this testing scenario the area of influence of 800°C brittleness zone. However, due to the fact that the material remained at this temperature for a relatively “short” time, this influence, albeit important, did not leave any meaningful traces of permanent character.

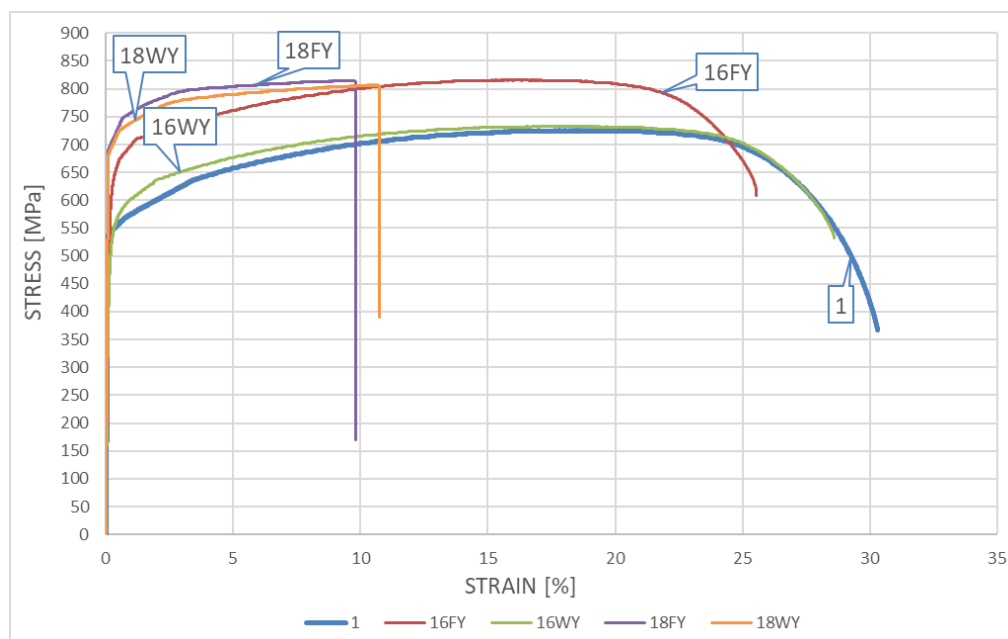


Figure 9. The σ - ε relationship obtained on samples made of $X2CrNiMoN22-5-3$ steel – a “long” fire scenario.

The relationships obtained during testing the influence of “long” fire on this steel grade are of similar character. Under each of the scenarios considered in detail here the tested steel hardened substantially. The hardening was significantly more pronounced, than the one observed on samples subjected to “short” fire. So long heating time proved to be the key quantitative difference, as it resulted in heavily expressed 800°C brittleness phenomenon. This phenomenon resulted in significant hardening of tested steel but at the same time disqualified it from extended service after surviving a fire incident. This is visible on graphs identified as 18FY and 18WY. This type of threat was not revealed during the simulation of “short” fire, as the one hour long heating time was too short to realize precipitation of deleterious precipitates related to the 800°C brittleness zone in amounts capable of affecting the properties of tested material. The higher post-fire ductility observed on samples tested following the 16WY scenario with respect to analogous ductility observed on samples tested following the 16FY scenario, associated with less pronounced hardening of tested material may be attributed to faster transition of the cooled sample through the 475°C brittleness zone.

Detailed juxtaposition of experimentally determined material parameters describing post-fire mechanical properties of X2CrNiMoN22-5-3 steel is listed in the Table 2, where:

- $A_t = \frac{\Delta L_t}{L_t} \cdot 100\%$ - denotes relative elongation of the sample, measured with respect to its total length L_t (Figure 2),
- $A_k = \frac{L_u - L_0}{L_0} \cdot 100\%$ - denotes relative elongation of the initial measurement base, measured on the broken sample (Figure 2).

Table 2. Post-fire mechanical properties of the X2CrNiMoN22-5-3 steel determined during static tensile test.

Sample identification	R _{0.2} [MPa]	R _m [MPa]	A _t [%]	A _k [%]
1	537	726	30.3	34.1
16FX	607	803	26.8	32.8
16WX	533	737	37.3	37.8
18FX	506	766	27.8	30.6
18WX	520	746	29.3	32.8
16FY	604	816	25.6	29.3
16WY	521	735	28.6	34.3
18FY	689	815	9.8	5.3
18WY	681	805	10.8	5.6

Due to the destruction mode of samples made of this steel grade, the cross-sectional area after breaking (S_u) could not be measured, and therefore it was impossible to determine the reduction in the area of cross-section Z [%].

5.3. Results obtained for the LDSS X2CrMnNiN21-5-1 steel

Appropriate σ - ε relationships, obtained during the tests on samples subjected to heating followed by cooling and made of X2CrMnNiN21-5-1 steel are depicted in Figure 10 – relating to the “short” fire scenario and in Figure 11 – relating to the “long” fire scenario.

Detailed analysis of both these graphs indicates relatively small influence of the simulated fire action on mechanical properties of the tested material determined experimentally post-fire. Both material strength and ductility in general seem to remain unaffected by the fire action regardless of fire duration applied (“short” fire versus “long” one). The quantitative differences observed seem to be negligible from the point of view of a potential designer striving to keep the analysed material in service after a fire incident, as these changes do not affect the capability of the material to safely resist the loads applied to it. It does not seem to be important, how the sample was cooled after surviving a fire incident, the temperature at which the sample has been kept, or for how long it has been kept

at such temperature. One may even dare to say, that under several of the testing scenarios followed, the post-fire mechanical properties of the tested material slightly improved when compared against the same properties determined on the samples in “as manufactured” condition, i.e. not affected by the simulated fire action.

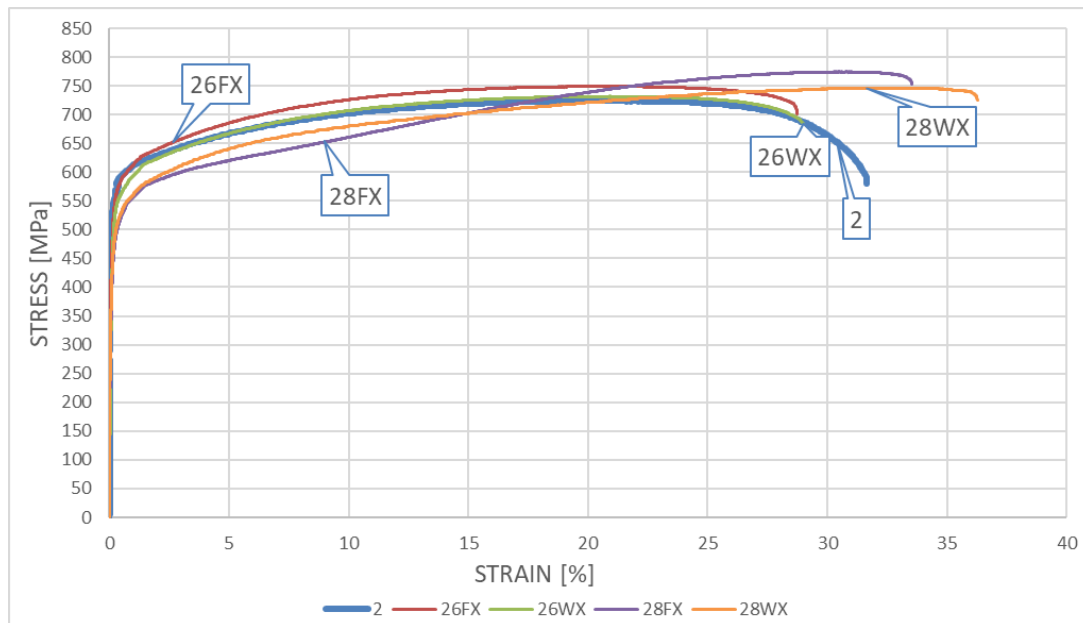


Figure 10. The σ - ϵ relationship obtained on samples made of $X2CrMnNiN21-5-1$ steel – a “short” fire scenario.

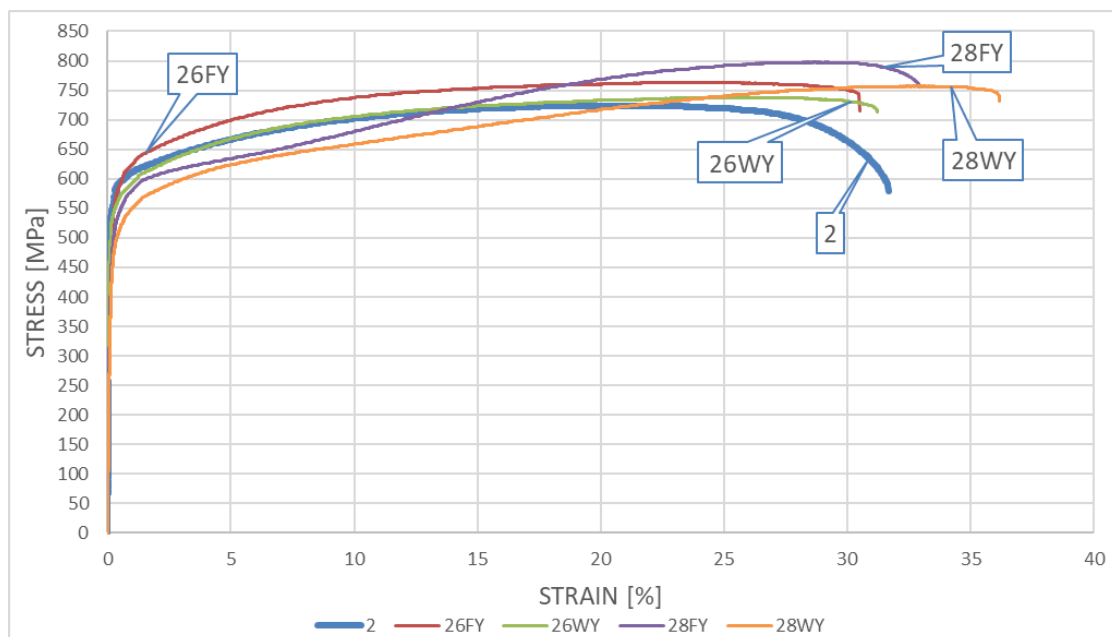


Figure 11. The σ - ϵ relationship obtained on samples made of $X2CrMnNiN21-5-1$ steel – a “long” fire scenario.

In general, the weak influence of fire incident duration and intensity on the registered post-fire strength and ductility clearly distinguishes the LDSS steel from the DSS 22% Cr steel described above. This is a consequence of a shift to the right on the TTT (*Time – Temperature – Transformation*) graph of the temperature range related to the 800°C brittleness. Thus, from the practical point of view, application of the LDSS steel seems to be safer when compared against the SDSS steel, if only the risk of prior exposure to fire should be considered.

Detailed juxtaposition of experimentally determined material parameters describing post-fire mechanical properties of $X2CrMnNiN21-5-1$ steel is listed in the Table 3. The sample destruction mode, analogous to the one observed on samples made of $X2CrNiMoN22-5-3$ steel, precluded the measurement of cross-section area after breaking. As a result the value of parameter Z [%], identifying the degree of sample necking, could not be determined.

Table 3. Post fire mechanical properties of the $X2CrMnNiN21-5-1$ steel determined during static tensile test.

Oznaczenie próbki	$R_{0.2}$ [MPa]	R_m [MPa]	A_t [%]	A_k [%]
2	528	724	31.7	37.0
26FX	497	749	28.8	33.5
26WX	513	732	29.0	33.4
28FX	433	774	33.6	33.9
28WX	468	746	36.3	38.3
26FY	529	760	30.5	33.4
26WY	511	740	31.2	33.8
28FY	485	800	33.0	34.5
28WY	461	760	36.2	38.8

6. Quantification of post-fire recovery coefficients observed on strength and ductility of tested steels

The coefficients quantifying permanent change in given quantity determined on the cooled material after a fire episode when compared against its initial value characterizing considered material before a simulated fire exposure will constitute an a posteriori recovery measure of strength and ductility observed on stainless steels tested here. Therefore the following quotients, in the professional bibliography identified as *retention factors* referring to, respectively: $R_{y,\Theta}^{post} = (f_{y,\Theta}^{post} / f_{y,20})$ – the yield limit, $R_{u,\Theta}^{post} = (f_{u,\Theta}^{post} / f_{u,20})$ – the ultimate tensile strength, $R_{E,\Theta}^{post} = (E_{a,\Theta}^{post} / E_{a,20})$ – the linear modulus of elasticity and $R_{\varepsilon_{u,\Theta}}^{post} = (\varepsilon_{u,\Theta}^{post} / \varepsilon_{u,20})$ – the limit strain of tested steel sample resulting in fracture will be used here. Detailed values of these factors obtained during the tests reported here are listed in Tables 4 and 5, separately for each steel grade tested. These results are also shown in Figure 12a–d for the $X2CrNiMoN22-5-3$ steel and below in Figure 13a–d for the $X2CrMnNiN21-5-1$ steel, with each testing result attributed to testing scenario indicated by a symbol explained in detail in the chapter 5.1 of this paper. The graphs mentioned here depict retention factors obtained experimentally in our research, related to their values recommended for practical application, juxtaposed in papers [35,36] for steels of duplex two-phase internal structure as a result of research by Molken and his team. For $X2CrMnNiN21-5-1$ steel, belonging to the LDSS group, this data, presented in Figure 13a-13d has been completed by alternative recommendations originating in [29], as these recommendations have been calibrated taking into account the specificity of this material. It has to be underlined, however, that all these values mentioned above and recommended for practical application are interpreted as appropriate quantiles of material properties interpreted as random variables. Thus the probability of their actual underestimation, accepted by the user of a given building that is to be used after a fire, is set at an intentionally low level.

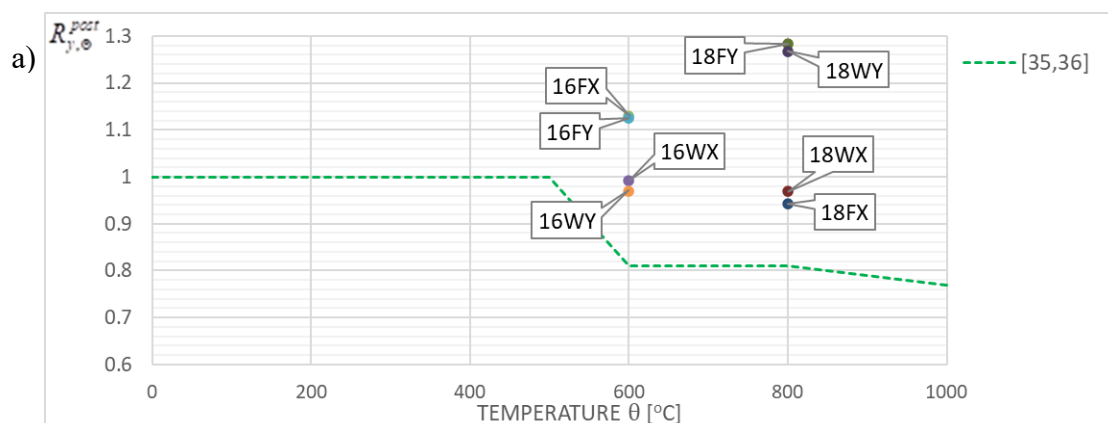
Table 4. Values of particular retention factors obtained experimentally for X2CrNiMoN22-5-3 steel belonging to the DSS 22% Cr class, based on the fire exposure scenario followed.

Fire exposure scenario followed during the experiment	$R_{y,\theta}^{post}$	$R_{u,\theta}^{post}$	$R_{E,\theta}^{post}$	$R_{\varepsilon u,\theta}^{post}$
16FX	1.13	1.11	1.05	0.88
16WX	0.99	1.02	1.01	1.23
18FX	0.94	1.06	0.96	0.92
18WX	0.97	1.03	0.99	0.97
16FY	1.12	1.12	1.04	0.84
16WY	0.97	1.01	0.99	0.94
18FY	1.28	1.12	1.08	0.32
18WY	1.27	1.11	1.07	0.36

Table 5. Values of particular retention factors obtained experimentally for X2CrMnNiN21-5-1 steel belonging to the LDSS class, based on fire exposure scenario followed.

Fire exposure scenario followed during the experiment	$R_{y,\theta}^{post}$	$R_{u,\theta}^{post}$	$R_{E,\theta}^{post}$	$R_{\varepsilon u,\theta}^{post}$
26FX	0.94	1.03	0.94	0.91
26WX	0.97	1.01	0.97	0.91
28FX	0.82	1.07	0.84	1.06
28WX	0.89	1.03	0.90	1.15
26FY	1.00	1.05	1.00	0.96
26WY	0.97	1.02	0.97	0.98
28FY	0.92	1.10	0.92	1.04
28WY	0.87	1.05	0.87	1.14

The yield limit of steel f_y is understood on these graphs as a conventional value, listed at the $R_{0.2}$ stress level. The value of limit strain ε_u has been determined based on the measured elongation A_t listed in detail in Tables 2 and 3, to preserve the compliance with scale used on horizontal axis in Figure 8-11.



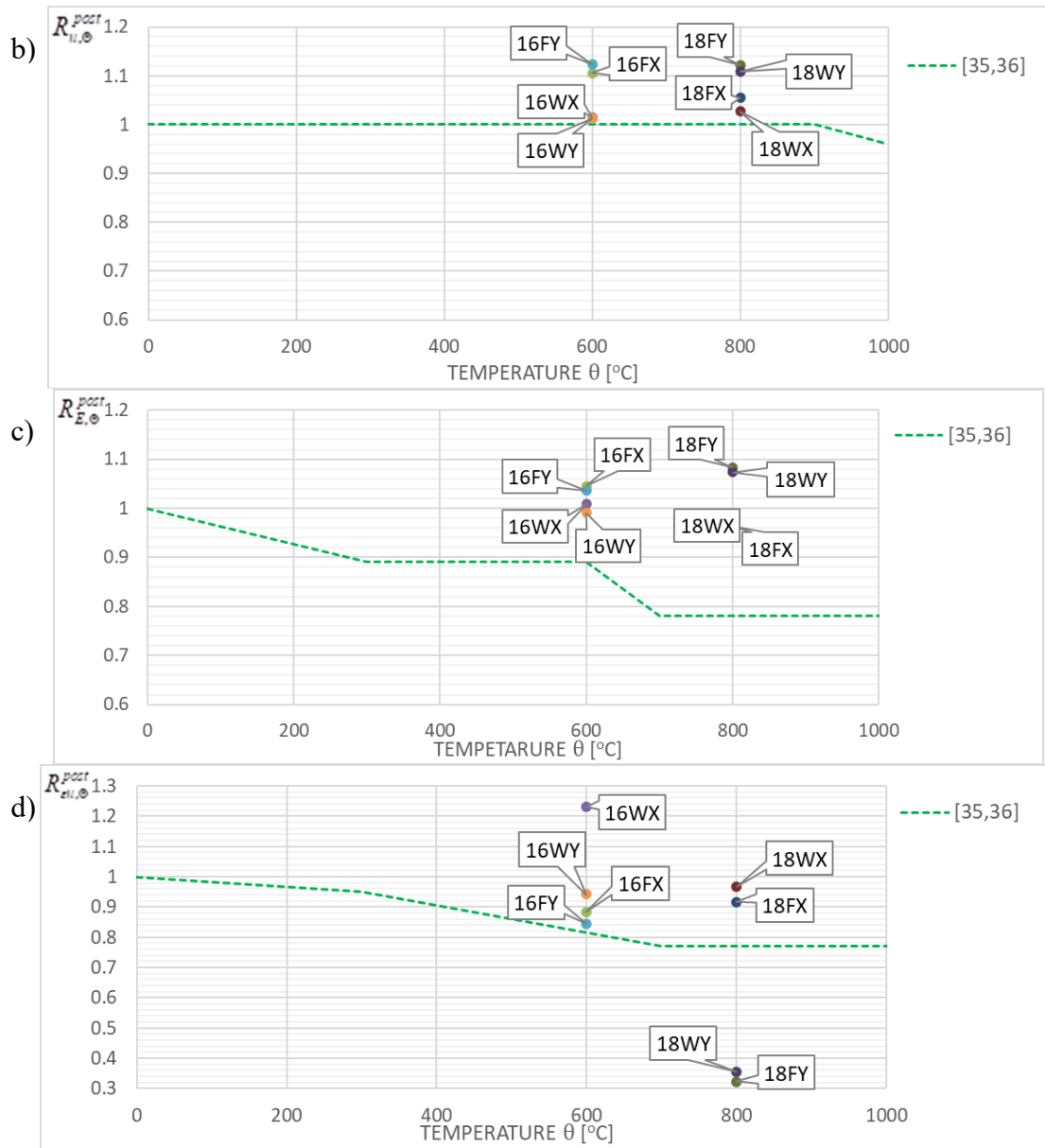
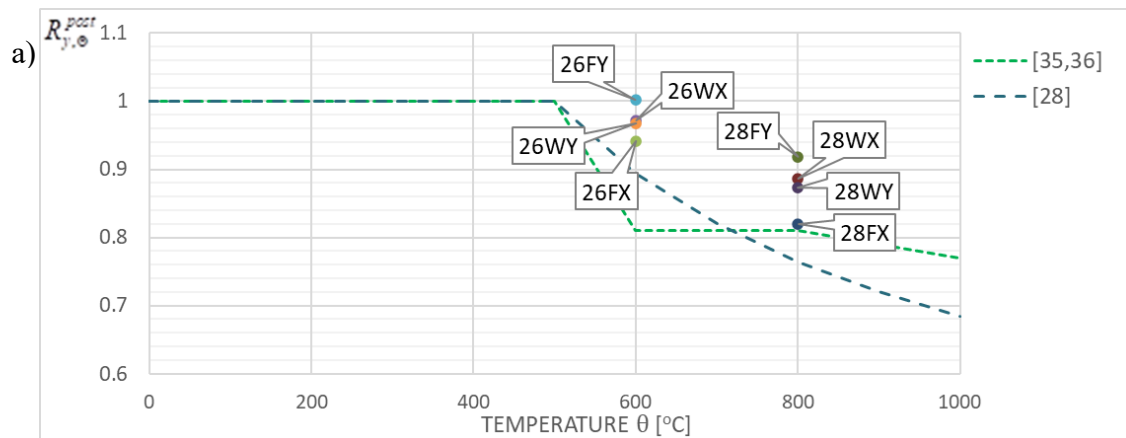


Figure 12. Post-fire strength and ductility retention factors obtained during tests on X2CrNiMoN22-5-3 stainless steel grade juxtaposed with their values recommended in [35,36].



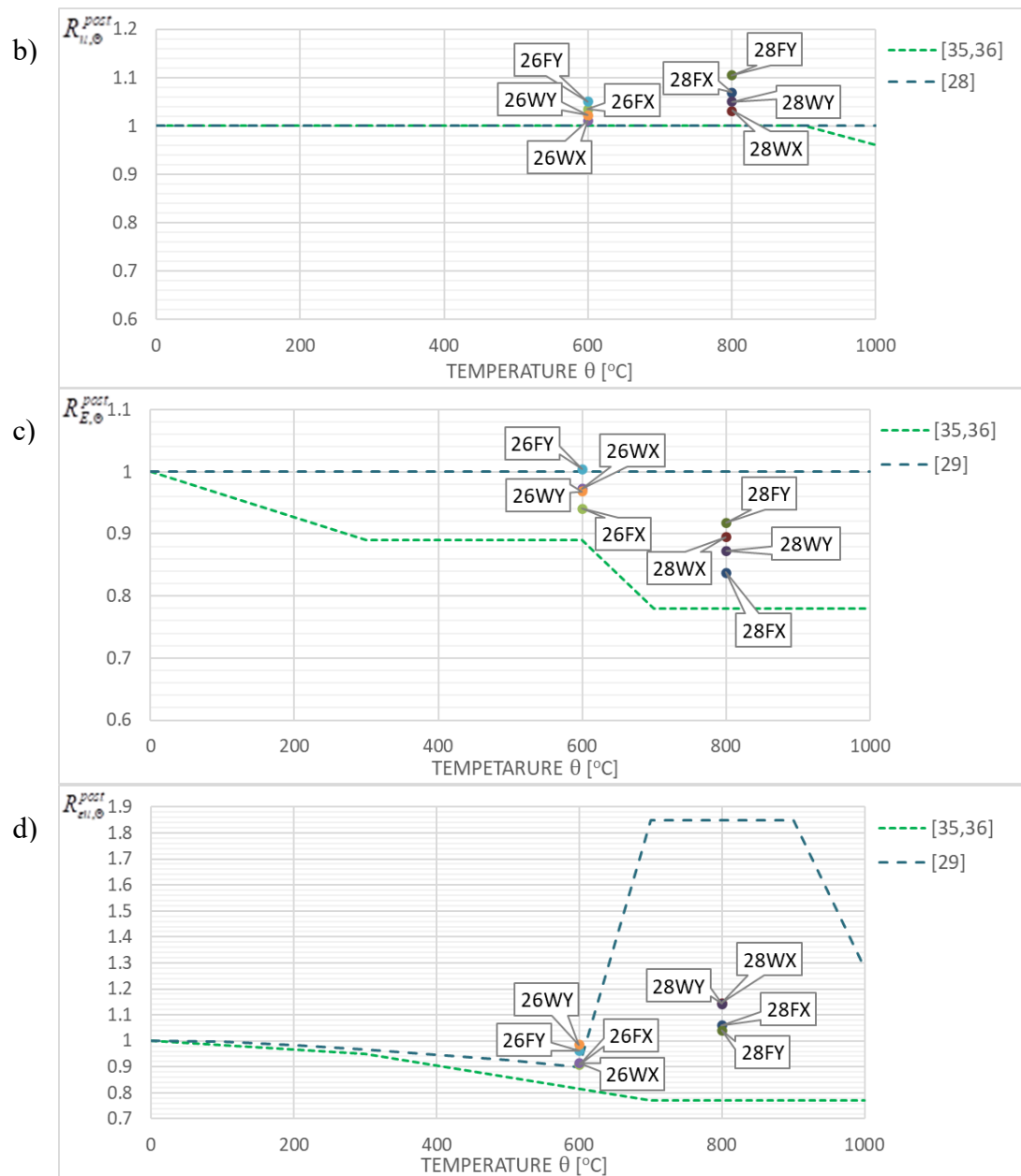


Figure 13. Post fire strength and ductility retention factors obtained during tests on X2CrMnNiN21-5-1 stainless steel grade juxtaposed with their values recommended in [29] as well as in [35,36].

The results of research reported here seem, to a large extent, positively conform to the recommendations contained in papers [35,36], as they are located in general on safe side when compared against the bottom limit values contained therein. This remark pertains not only to the post-fire strength of tested steel but also to its post-fire ductility. A significantly reduced capacity for plastic strain exhibited by a sample previously heated for a sufficiently long time at the temperature of 800°C, regardless of the cooling mode applied (as shown for samples denoted as 18FY and 18WY in Figure 12d, and, independently, in Figure 9) seems to be a notable exception here. Such behavior, due to the inherent risk of brittle failure unequivocally excludes the capacity for extended use of this steel grade and in particular for safe load-bearing service after a fire. It is clearly visible, that in many fire development scenarios, which may occur in real life, the tested steel does not fully recover its strength after surviving a fire incident followed by effective cooling. However, in many other scenarios mechanical properties of this steel visibly improve, and this may justify the absence of recommendation to apply an appropriate reduction in limit tensile strength value and an assumption of $R_{u,\theta}^{post} = 1.0$ as depicted in Figure 12b and 13b. In this context the recommendation contained in

[35,36] pertaining to the reduction in the value of linear elasticity coefficient (Figure 12c and 13c) seems to be very safe as well.

However, our research reported here did not confirm so clear improvement in the post-fire ductility exhibited by the X2CrMnNiN21-5-1 steel belonging to the LDSS group and effectively cooled after a simulated fire exposure at 800°C, as the authors of paper [29] (Figure 13c) would like to see.

An alternative approach to describe the dependence of post-fire steel mechanical properties reduction factor on the maximum temperature at which the steel had been heated has been proposed in paper [36]. The reduction of i -th property in this approach is still measured by the retention coefficient $R_{i,\Theta}^{post}$, but this time it is interpreted as a product of the code reduction coefficient $k_{y,\tau}$, $k_{u,\tau}$ and $k_{E,\tau}$ respectively, determined as for the fire conditions based on the Appendix C to the code [37], and a corresponding recovery factor $r_{y,\Theta}^{post}$, $r_{u,\Theta}^{post}$ and $r_{E,\Theta}^{post}$, specified for post-fire conditions. The recovery factors $r_{i,\Theta}^{post}$ in this method are determined as a quotient of i -th value determined on a sample cooled after fire and appropriately reduced value of the same property determined at given heating time under assumption of fire scenario. Therefore, for example

$$R_{y,\Theta}^{post} = \frac{f_{y,\Theta}^{post}}{f_{y,20}} = k_{y,\Theta} r_{y,\Theta}^{post} = \frac{f_{y,\Theta}^{fire}}{f_{y,20}} \frac{f_{y,\Theta}^{post}}{f_{y,\Theta}^{fire}}. \text{ Juxtaposition of recovery factors obtained during research}$$

presented here accompanied by the information on sample heating and cooling scenarios followed during the tests is presented in Tables 6 and 7.

Table 6. Values of particular recovery factors obtained experimentally for X2CrNiMoN22-5-3 steel, based on the fire exposure scenario followed.

Fire exposure scenario followed during the experiment	$r_{y,\Theta}^{post}$	$r_{u,\Theta}^{post}$	$r_{E,\Theta}^{post}$
16FX	2.69	1.98	1.38
16WX	2.36	1.81	1.33
18FX	6.28	4.80	1.53
18WX	6.46	4.67	1.57
16FY	2.68	2.01	1.36
16WY	2.31	1.81	1.30
18FY	8.55	5.10	1.72
18WY	8.45	5.04	1.70

Table 7. Values of particular recovery factors obtained experimentally for X2CrMnNiN21-5-1 steel, based on the fire exposure scenario followed.

Fire exposure scenario followed during the experiment	$r_{y,\Theta}^{post}$	$r_{u,\Theta}^{post}$	$r_{E,\Theta}^{post}$
26FX	2.24	1.85	1.24
26WX	2.31	1.81	1.28
28FX	5.47	4.86	1.33
28WX	5.91	4.68	1.42
26FY	2.39	1.87	1.32
26WY	2.30	1.83	1.27
28FY	6.12	5.02	1.46
28WY	5.82	4.77	1.39

7. Concluding remarks

The research conducted by us has shown quite significant differences in post-fire mechanical properties exhibited by the stainless steel grades compared. In spite of the fact, that the X2CrNiMoN22-5-3 steel, belonging to the LDSS class due to its intentionally selected chemical

composition, is usually treated as a cheaper substitute of the X2CrNiMoN22-5-3 steel belonging to the DSS 22% Cr class, it proved to be more resistant in the context of post-fire brittleness phenomenon. A risk of this type is, for stainless steels exhibiting two-phase austenitic-ferritic microstructure of duplex type, particularly important. This is usually associated with the potential capacity to reveal harmful precipitates in the structure of the material, negatively affecting its strength and ductility. This precipitation process usually intensifies in two temperature ranges, at high temperature – the so called 800°C brittleness, and at lower temperature – the so called 475°C brittleness, respectively. The extent to which passing through both these ranges would permanently weaken given steel grade during cooling process and persist after the steel attained the ambient temperature depends mostly on the time spent passing through both these ranges. Therefore rapid cooling of tested samples in water mist, significantly speeding up the cooling process, usually proves to be more advantageous, when compared against traditional slow cooling in the muffle furnace, modeling self-extinguishing of the fire. However, overly rapid cooling may result in local hardening of the material. This will result in strengthening, but at the expense of substantial increase in susceptibility to brittle failure.

It has to be underlined, that during the research reported here the 800°C brittleness zone was reached only when following the scenarios, where the tested samples were heated at the temperature enabling austenitic transformation. When following the “short” fire scenario the samples were kept at this temperature for a time too short to fully reveal the deleterious phenomena related to the 800°C brittleness. Only after this time has been extended to 10 hours, as in the “long” fire scenario, it was possible to observe full scale results of the coincidence of both phenomena described above.

Of all testing scenarios applied during our research, only those denoted as 18FY and 18WY resulted in material state after simulated fire excluding its capacity to safely resist the loads applied. Let us note, however, that under those scenarios the cooling mode applied proved to be unimportant. The full scale brittleness revealed itself in those cases as a direct result of heating to sufficiently high temperature, initiating austenitic transformation in its internal structure, followed by keeping the hot material at this temperature for a time long enough to fully reveal the deleterious results of 800°C brittleness phenomenon.

Both the retention factor values $R_{i,\Theta}^{post}$, listed in this paper in the Tables 4 and 5, as well as the correlated values of recovery factors $r_{i,\Theta}^{post}$, listed here in the Tables 6 and 7 allow the potential designer, deciding whether to further use a given steel grade to safely support the loads applied to it after fire, the opportunity to assess to what extent this steel grade retained its initial strength and ductility under those circumstances. Of course, the values assigned to recovery factors in this approach are substantially higher than those assigned to retention factors, as the former refer to material properties significantly deteriorated in fire temperature, instead of those characterizing it prior to fire incident and thus not subject to reduction.

Detailed analysis of these factors determined during our experiments leads to the conclusion, that for both steel grades tested the ultimate strength $f_{u,\Theta}^{post}$ determined after simulated fire, in each of tested simulated fire scenario episodes was not only fully preserved, but even slightly increased. This conclusion does not hold in the case of conventional yield limit $f_{y,\Theta}^{post}$, but the relative reduction of this value, as determined during the experiment, does not seem to be computationally relevant. Let us note, however, that when this evaluation criterion is applied, the degree to which it is permanently reduced, determined on the material cooled after surviving an a priori fire incident, seems to be more pronounced when a steel grade belonging to the LDSS is considered.

In general, the $R_{i,\Theta}^{post}$ retention factor values close to 1.0 obtained during our experiments on both steel grades indicate that the possible further use of these steel grades after fire is restricted by a qualitative understanding of the possible risk of full scale brittleness effects being revealed in the material, as the risk of brittle failure seems to constitute the critical factor here. As shown above, such critical scenario has been identified and discussed. The value of $R_{\epsilon u,\Theta}^{post}$ related to this scenario, indicates that only 32-36% of the initial material ductility has been recovered (Table 4). Interestingly,

as mentioned earlier, this type of threat was not identified for steel classified as LDSS (Table 5) in the fire scenarios analyzed.

Figures 12 and 13 included in this paper confirm, that the recommendations presented in papers [35,36] and interpreted as appropriate quantiles of particular values treated as random variables have been calibrated generally in a safe way, though, in certain cases in a somewhat risky manner. The proposals for such calibrations found in [29] seem not to be confirmed in the view of our research. This is visible in particular in Figure 13d.

References

1. Boniardi M., Casaroli A. *Stainless steels*, Politecnica di Milano, Dipartimento di Meccanica, Gruppo Lucefin Research and Development, Esine, Brescia, Italy, 2014.
2. *Practical guidelines for the fabrication of duplex stainless steel*, Second Edition, International Molybdenum Association (IMOA), London, UK, 2009.
3. *Practical guide to using duplex stainless steels. A guide to the use of nickel-containing alloys No 10044*, Nickel Institute, 2020, www.nickelinstitute.org.
4. *Design manual for structural stainless steel*, 4th edition, SCI Publication P413, SCI, Silwood Park, Ascot, UK, 2017, www.steel-sci.com.
5. Maslak M., Stankiewicz M., Slazak B. Duplex steels used in building structures and their resistance to chloride corrosion, *Materials*, 14, 2021, 5666, <https://doi.org/10.3390/ma14195666>.
6. Nilsson J.O. Super duplex stainless steels, *Materials Science and Technology*, vol. 8, 1992, 685-700, <https://doi.org/10.1179/mst.1992.8.8.685>.
7. Chai Guocai, Kangas Pasi. Super and hyper duplex stainless steels: structures, properties and applications, 21st European Conference on Fracture (ECF 21), June 20-24, Catania, Italy, *Procedia Structural Integrity* 2, 2016, 1755-1762, <https://doi.org/10.1016/j.prostr.2016.06.221>.
8. Liljas M., Johansson P., Liu H.P., Olsson C.A. Development of a lean duplex stainless steel. *Steel Research International*, 79, 2008, 466e73.
9. EN 10088-1, *Stainless steels, Part 1: List of stainless steels*, 2014.
10. Li Xiang, Lo K.H., Kwok C.T., Sun Y.F., Lai K.K. Post-fire mechanical and corrosion properties of duplex stainless steel: comparison with ordinary reinforcing-bar steel, *Construction and Building Materials*, 174, 2018, 150–158, <https://doi.org/10.1016/j.conbuildmat.2018.04.110>.
11. Escobar J.D., Delfino P.M., Ariza-Echeverri E.A., Carvalho F.M., Schell N., Stark A., Rodrigues T.A., Oliveira J.P., Avila J.A., Goldenstein H., Tschiptschin A.P. Response of ferrite, bainite, martensite, and retained austenite to a fire cycle in a fire-resistant steel, *Materials Characterization*, 182, 2021, 111567, <https://doi.org/10.1016/j.matchar.2021.111567>.
12. Pancikiewicz K., Maslak M., Pazdanowski M., Stankiewicz M., Zajdel P. Changes in the microstructure of selected structural alloy steel grades identified after their simulated exposure to fire temperature, *Case Studies in Construction Materials*, 18, 2023, e01923, <https://doi.org/10.1016/j.cscm.2023.e01923>.
13. Bednarek Z., Kamocka R. The heating rate impact on parameters characteristic of steel behaviour under fire conditions, *Journal of Civil Engineering and Management*, XII, 4, 2006, 269-275, <https://doi.org/10.3846/13923730.2006.9636403>.
14. Maslak M., Pazdanowski M., Stankiewicz M., Zajdel P. Influence of the duration of fire incidents on the post-fire impact strength of selected steel grades used in constructions, 4th International Fire Safety Symposium (IFireSS), Rio de Janeiro, Brazil, June 21-23, 2023, 623-636.
15. Maslak M., Zwirski G. Changes in structural steel microstructures following heating and cooling episodes in fires, *Safety & Fire Technique*, 48, 2017, 34-52.
16. Topolska S., Łabanowski J. Impact-toughness investigations of duplex stainless steels, *Materiale i Tehnologije / Materials and Technology*, 49, 4, 2015, 481–486, <https://doi.org/10.17222/mit.2014.133>.
17. EN-ISO 148-1 *Metallic materials – Charpy pendulum impact test. Part 1: Test method*, 2006.
18. EN-ISO 14556 *Metallic materials – Charpy V-notch pendulum impact test. Instrumented test method*, 2015.
19. ASTM E 23-92 *Standard test methods for notched bar impact testing of metallic materials*, 2016.
20. ASTM E 2298-18 *Standard test method for instrumented impact testing of metallic materials*, 2018.
21. Zajdel, P. Interpretation of the results of the impact test performed with an instrumented Charpy pendulum for the purposes of assessing the properties of structural steels. *Inżynieria i Budownictwo*, 7, 2020, 341–344, (in Polish).
22. Zajdel, P. A suitability assessment using an instrumented impact test of the use of selected structural steel grades on the basis of their changes in response to exposure to fire. *Technical Transactions*, 118, 2021, e2021007.

23. Maslak M., Pazdanowski M., Stankiewicz M., Zajdel P. The impact strength of selected steel types after fire. Experimental tests related to simulated fire conditions, in: Pečenko R., Huč S., Chifliganec C., Hozjan T. (Eds.) Applications of Structural Fire Engineering (ASFE), Proceedings of the International Conference, Ljubljana, Slovenia, June 10-11, 2021, University of Ljubljana, 2021, 55-60.
24. Maslak M., Pazdanowski M., Stankiewicz M., Zajdel P. Post-fire susceptibility to brittle fracture of selected steel grades used in construction industry – assessment based on the instrumented impact test, *Materials*, 14, 2021, 3922, <https://doi.org/10.3390/ma14143922>.
25. Maslak M., Stankiewicz M., Zajdel P. The brittleness of selected structural steel grades specified after a simulated fire exposure, *Inżynieria i Budownictwo*, 5-6, 2022, 232-239, (in Polish).
26. Maslak M., Pazdanowski M., Stankiewicz M., Wassilkowska A., Zajdel P., Zielina M. Impact fracture surfaces as the indicators of structural steel post-fire susceptibility to brittle cracking”, *Materials*, 16, 8, 2023, 3281, <https://doi.org/10.3390/ma16083281>.
27. Maslak M., Pazdanowski M., Stankiewicz M., Zajdel P. Testing of post-fire brittleness for certain standard duplex and lean duplex stainless steel grades, *ce/papers*, 6, 3-4, 2023, 514-519, <https://doi.org/10.1002/cepa.2356>.
28. Laukkanen A., Uusikallio S., Lindroos M., Andersson T., Kömi J., Porter D. Micromechanics driven design of ferritic–austenitic duplex stainless steel microstructures for improved cleavage fracture toughness, *Engineering Fracture Mechanics*, 253, 2021, 107878, <https://doi.org/10.1016/j.engfracmech.2021.107878>.
29. Huang Yun'er, Young B. Mechanical properties of lean duplex stainless steel at post-fire condition, *Thin-Walled Structures*, 130, 2018, 564–576, <https://doi.org/10.1016/j.tws.2018.06.018>.
30. Xie Li-An, Wang Xing-Qiang, Han Zhi-Jiang, Yu Xin, Alim M.A., Manninen T., Tao Zhong Post-fire stress-strain response of structural ferritic stainless steels, *Journal of Constructional Steel Research*, 196, 2022, 107389, <https://doi.org/10.1016/j.jcsr.2022.107389>.
31. Jie Lu, Hongbo Liu, Zhihua Chen, Xiangwei Liao Experimental investigation into the post-fire mechanical properties of hot-rolled and cold-formed steels, *Journal of Constructional Steel Research*, 121, 2016, 291–310, <https://doi.org/10.1016/j.jcsr.2016.03.005>.
32. Gunalan Shanmuganathan, Mahendran Mahen Experimental investigation of post-fire mechanical properties of cold-formed steels, *Thin-Walled Structures*, 84, 2014, 241–254, <http://dx.doi.org/10.1016/j.tws.2014.06.010>.
33. ISO 6892-1 Metallic materials – Tensile testing - Part 1: Method of test at room temperature, Third edition, 2019.
34. Maslak M., Pazdanowski M., Stankiewicz M., Zajdel P. Testing of post-fire resistance for selected stainless steel grades used in construction, Proceedings of the 11th International Conference on Advances in Steel Structures (ICASS), December 5-7, 2023, Kuching, Sarawak, Malaysia.
35. Molkens T., Cashell K.A., Malaska M., Alanen M., Rossi B. Post-fire behaviour of structural stainless steel, *ce/papers* 4, 2-4, 2020, 1411-1420.
36. Molkens T., Cashell K.A., Malaska M., Alanen M., Rossi B. Performance of structural stainless steel following a fire, *Engineering Structures*, 235, 2021, 112001.
37. EN 1993-1-2 Eurocode 3: Design of steel structures, Part 1-2 General rules – structural fire design, Annex C: Stainless steels.

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